

THIS IS A 3-PAGE FORM - ALL INFORMATION IS REQUIRED FOR PROJECT REVIEW

HISTORIC DISTRICT COMMISSION PROJECT REVIEW REQUEST

City of Detroit - Planning & Development Department
2 Woodward Avenue, Suite 808
Detroit, Michigan 48226

Date: 06/03/2020

PROPERTY INFORMATION

ADDRESS: 867 Edison Street

AKA: _____

HISTORIC DISTRICT: Boston-Edison

SCOPE OF WORK: (Check ALL that apply)

| | | | | |
|--|---|---|--|---|
| <input type="checkbox"/> Windows/ Doors | <input type="checkbox"/> Roof/Gutters/ Chimney | <input type="checkbox"/> Porch/ Deck | <input type="checkbox"/> Landscape/Fence/ Tree/Park | <input type="checkbox"/> General Rehab |
| <input type="checkbox"/> New Construction | <input type="checkbox"/> Demolition | <input type="checkbox"/> Addition | <input checked="" type="checkbox"/> Other: <u>Garage</u> | |

APPLICANT IDENTIFICATION

Property Owner/
Homeowner Contractor Tenant or
Business Occupant Architect/Engineer/
Consultant

NAME: George Bogaert

COMPANY NAME: Tuff Shed Inc.

ADDRESS: 34425 Schockcraft Rd.

CITY: Livonia

STATE: MI

ZIP: 48150

PHONE: 734-853-5727

MOBILE: 586-804-9573

EMAIL: gbogaert@tuffshed.com

PROJECT REVIEW REQUEST CHECKLIST

Please attach the following documentation to your request:

PLEASE KEEP FILE SIZE OF ENTIRE SUBMISSION UNDER 30MB

Completed Building Permit Application (highlighted portions only)

ePLANS Permit Number (only applicable if you've already applied for permits through ePLANS)

Photographs of ALL sides of existing building or site

Detailed photographs of location of proposed work (photographs to show existing condition(s), design, color, & material)

Description of existing conditions (including materials and design)

Description of project (if replacing any existing material(s), include an explanation as to why replacement--rather than repair--of existing and/or construction of new is required)

Detailed scope of work (formatted as bulleted list)

Brochure/cut sheets for proposed replacement material(s) and/or product(s), as applicable

NOTE:

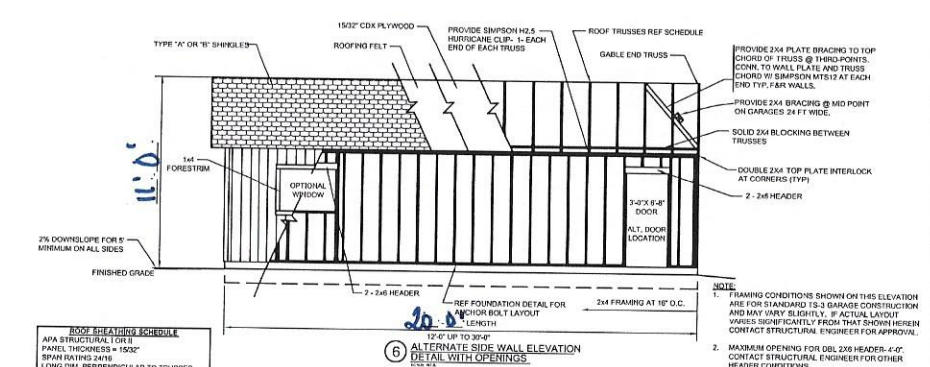
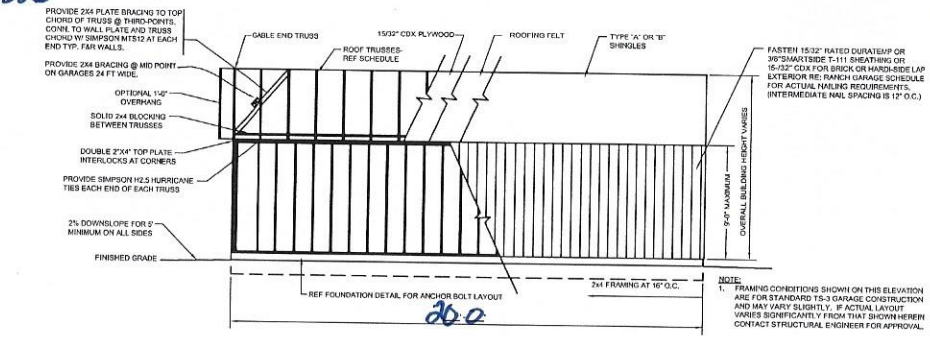
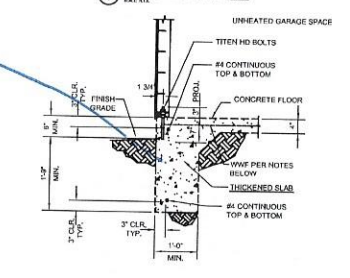
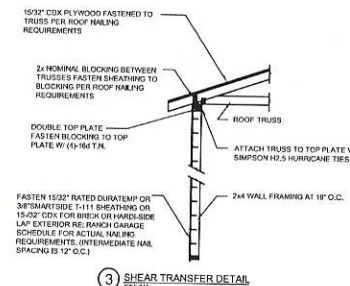
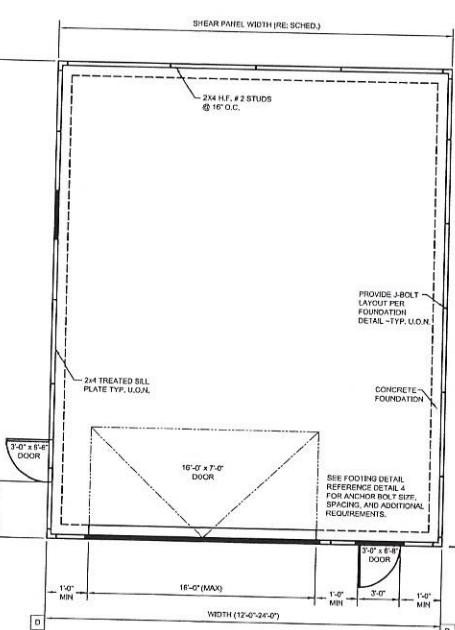
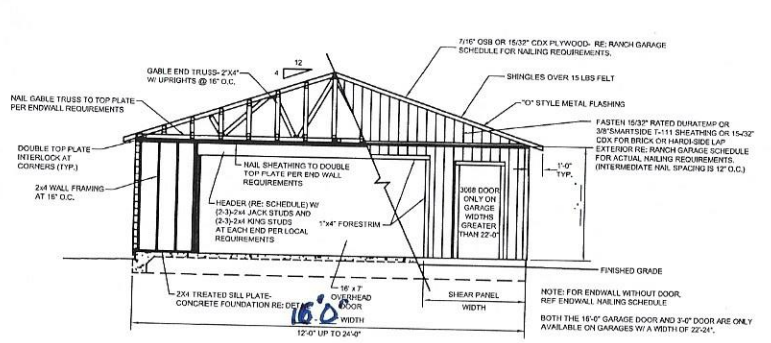
Based on the scope of work, additional documentation may be required.

See www.detroitmi.gov/hdc for scope-specific requirements.

Upon receipt of this documentation, staff will review and inform you of the next steps toward obtaining your building permit from the Buildings, Safety Engineering and Environmental Department (BSEED) to perform the work.

SUBMIT COMPLETED REQUESTS TO HDC@DETROITMI.GOV

RANCH GARAGE 12'-24' x 12'-30'
16' x 20' = 320 SQ FT



ROOF SHEATHING SCHEDULE

| | |
|-----------------------|--------------------------|
| APA STRUCTURAL LUMBER | 1/2" CONTINUOUS |
| PANEL THICKNESS | 1502" |
| SPAN BOWING | 24" B |
| LONG DIM. | PERPENDICULAR TO TRUSSES |

STRUCTURAL NOTES:

- REFERENCE: 2008 IRC WITH AMENDMENTS PER LOCAL BUILDING DEPARTMENT HAVING JURISDICTION OVER THIS PROJECT.
- DESIGN LOADING:**
WIND SPEED & EXPOSURE: 100
ROOF LIVE LOAD: 20 PSF
ROOF DEAD LOAD: 10 PSF
- LUMBER:**
1. ALL LUMBER SHALL BE 184-FR GRADE NO. 2 OR BETTER, WITH A BASE MINIMUM ALLOWABLE EXTREME FIBER BENDING STRESS FOR MEMBERS 2x4 OF 1600 P.S.I. (EXCEPT WHERE OTHERWISE SPECIFIED FOR USE, SIZE, LOAD DURATION, ENVIRONMENT, ETC., UNLESS OTHERWISE SPECIFIED).
- REFER TO THE TRUSS DESIGN FOR DESIGN INFORMATION.
- ROOFING:**
1. 20 YEAR FIBERGLASS SHINGLES (TYPE 'A' OR TYPE 'B')
- 15 LB ROOFING FELT
- TYPE 'M' METAL FLASHING AND DRIP EDGES REQUIRED ALL SIDES.

GENERAL:

- ERECTION PROCEDURES SHALL CONFORM TO OSHA STANDARDS. BUILDER SHALL PROTECT ALL ADJACENT PROPERTY, STRUCTURES, STREETS, UTILITIES, ETC.
- BUILDER IS RESPONSIBLE FOR THE SAFETY OF BUILDING DURING CONSTRUCTION. INCLUDE SHORING OR BRACING AS REQUIRED AND PER GOVERNING REGULATIONS.

RANCH GARAGE SCHEDULE

| OVERALL DIMENSIONS | SHEAR PANEL NAILING | ROOF SHEATHING | END HEADER | CHORD SPLICE NAILING |
|--------------------|---------------------|----------------|-------------|----------------------|
| WIDTH | MAX. LENGTH | MIN. LENGTH | MAX. HEIGHT | MIN. HEIGHT |
| 12'-0" | 24'-0" | 12'-0" | 8'-0" | 8'-0" |
| 12'-0" | 30'-0" | 28'-0" | 9'-0" | 8'-0" |
| 14'-0" | 30'-0" | 14'-0" | 9'-0" | 8'-0" |
| 18'-0" | 30'-0" | 18'-0" | 9'-0" | 8'-0" |
| 18'-0" | 28'-0" | 18'-0" | 9'-0" | 8'-0" |
| 20'-0" | 30'-0" | 20'-0" | 9'-0" | 8'-0" |
| 22'-0" | 30'-0" | 22'-0" | 9'-0" | 8'-0" |
| 24'-0" | 30'-0" | 24'-0" | 9'-0" | 8'-0" |

NOTES:

- ON SIDE WALLS PROVIDE A MINIMUM OF 8" COMBINED FULL HEIGHT SHEAR WALLS, WITH FULL HEIGHT SHEAR WALL SECTIONS NOT TO BE LESS THAN 24"
- NO OPENINGS ON THE OPPOSITE END WALLS OF THE GARAGE DOOR.
- MAX. THE END WALL WITH THE GARAGE DOOR OPENING WITH 16 @ 8" O.C.
- ALL NAILS USED SHOULD BE COMMON NAILS U.O.N.

Concrete by others

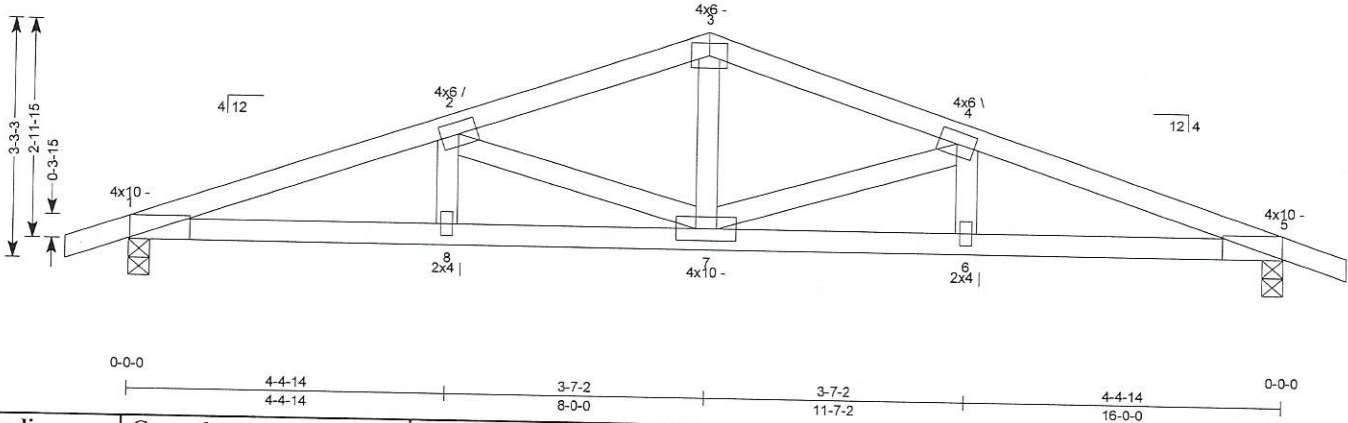
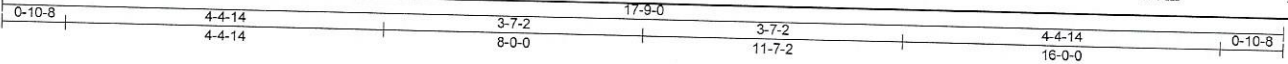
TUFF SHED
Storage Buildings & Garage

Customer: _____
Date: 10/29/08
Checked By: _____
Scale: N.T.S.

P.O.# _____
Drawn By: SAH
Date: 10/29/08
Checked By: _____
Scale: N.T.S.

Richard Weilgratt Consultants, INC.
STANDARD RANCH GARAGE END WALL OVERHEAD DOOR
SHEET 01
SHEET 1 OF 1

| | | | | | | | | | |
|----------------|---------------|-----------|---------------|---------------|-----------------|-----------------|-----------|------------------|-------------------|
| SPAN 16-0-0 | PITCH 4/12 | QTY 10 | OHL 0-10-8 | OHR 0-10-8 | CANT L 0-0-0 | CANT R 0-0-0 | PLYS 1 | SPACING 24 in | WGT/PLY 56 lbs |
|----------------|---------------|-----------|---------------|---------------|-----------------|-----------------|-----------|------------------|-------------------|



| Loading | General | CSI Summary | Deflection | L/ | (loc) | Allowed |
|--|--|---|--|--------------------|---------------------|--------------------|
| Load (psf) TCLL: 105 TCDL: 10 BCLL: 0 BCDL: 10 | Bldg Code: IRC 2006/ TPI 1-2002 Rep Mbr Increase: Yes D.O.L.: 11.5% | TC: 0.94 (1-2) BC: 0.93 (8-1) Web: 0.42 (2-7) | Vert TL: 0.35 in Vert LL: 0.29 in Horz TL: 0.12 in | L / 524 L / 629 | (6-7) (6-7) 5 | L / 180 L / 240 |

Reaction Summary

| JT | Type | Brg Combo | Brg Width | Rqd Brg Width | Max React | Max Grav Uplift | Max MWFRS Uplift | Max C&C Uplift | Max Uplift | Max Horiz |
|----|--------------|-----------|-----------|---------------|-----------|-----------------|------------------|----------------|------------|-----------|
| 1 | Pin (Wall) | 1 | 3.5 in | 3.62 in | 2,201 lbs | | | | | |
| 5 | HRoll (Wall) | 1 | 3.5 in | 3.62 in | 2,201 lbs | | -59 lbs | -51.5 lbs | -51.5 lbs | 6 lbs |

Bearing enhancers may be required at the following bearings:

| Brg # | Brg Area | Rqd Brg Area | Rqd Truss Width |
|-------|----------------------|----------------------|-----------------|
| 1 | 5.25 in ² | 5.44 in ² | 1.55 in |
| 5 | 5.25 in ² | 5.44 in ² | 1.55 in |

| Material Summary | | Bracing Summary | |
|------------------|-------------|-----------------|---|
| TC | HF #2 2 x 4 | TC Bracing | Sheathed |
| BC | HF #2 2 x 4 | BC Bracing | Sheathed or purlins at 72" OC, Purlin design by Others. |
| Webs | HF #2 2 x 4 | | |

Loads Summary

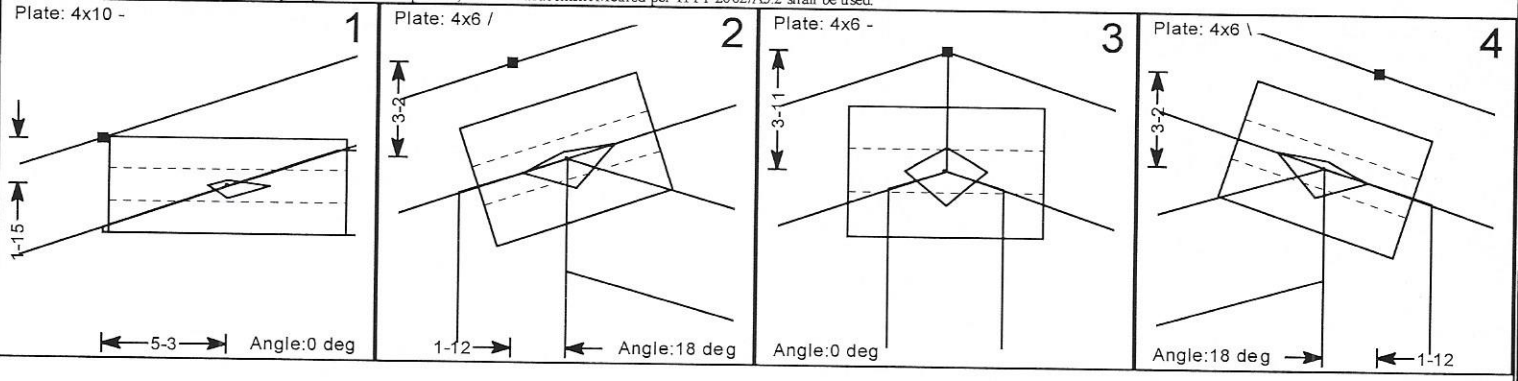
1) This truss has been designed for the effects of wind loads in accordance with ASCE 7 - 05 with the following user defined input: 105 mph, Exposure C, Enclosed, Cable/Hip, Building Category II (I = 1.00), Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
 2) Unbalanced roof live loads have not been considered.
 3) Minimum storage attic loading has been applied in accordance with IRC 301.5

Member Forces Summary

Table indicates Member ID, max CSI, max axial force, (max compr. force if different from max axial force)

| TC | BC | Webs |
|--|--|---|
| 9-1 0.170 64 lbs 1-2 0.935 -4,743 lbs | 2-3 0.755 -3,265 lbs 3-4 0.755 -3,265 lbs 5-6 0.933 4,387 lbs (-818 lbs) | 6-7 0.931 4,391 lbs (-816 lbs) 7-8 0.931 4,391 lbs (-816 lbs) 2-8 0.030 154 lbs 2-7 0.418 -1,555 lbs |

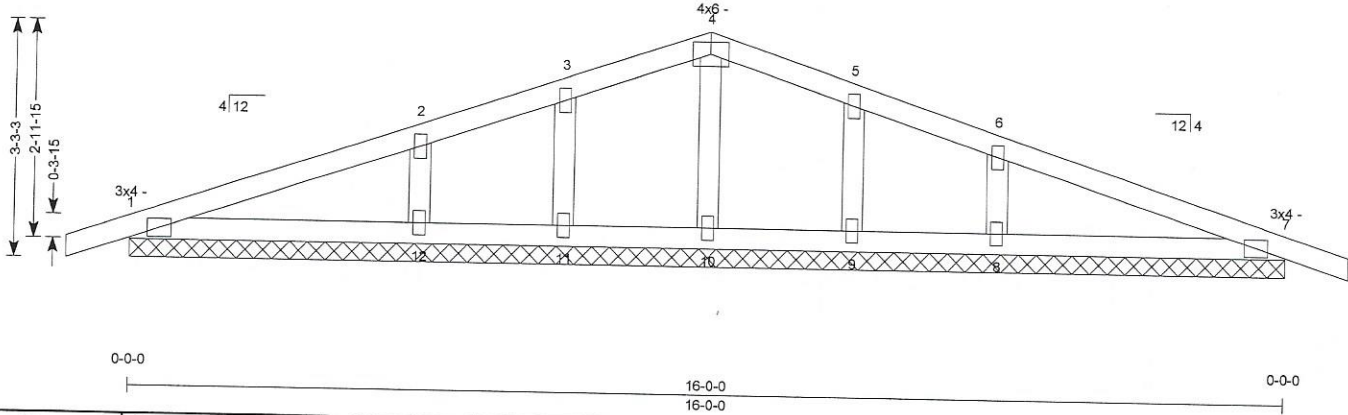
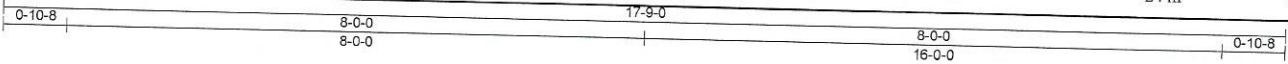
Notes:
 4) When this truss has been chosen for quality assurance inspection, the Plate Placement Method per TPI 1-2002/A3.2 shall be used.



A copy of this design shall be furnished to the erection contractor. This design is for an individual building component (a truss). It is based on specifications provided by the Truss Designer and performed in accordance with TPI 1-2002 and the 2001 NDS design standard. No responsibility is assumed for the accuracy of information provided by the Truss Designer. Dimensions shall be verified by building designer. Creep deflection is not automatically accounted for by the software. The building designer shall review loading, truss configuration and initial deflection data shown to ensure that this design meets or exceeds minimum loads required by applicable building codes. Compression chords shall be laterally braced by the roof or floor sheathing, directly attached, unless otherwise noted. Bracing shown is for lateral support of individual truss components only to reduce buckling length. It is not wind or lateral load bracing or overall building design bracing which is by others. Refer to BC S1-B3 for recommended truss handling and erection. Do not apply loads beyond weight of erectors until all permanent bracing is in place. Concentration of construction loads greater than the design loads shall not be applied to the trusses at any time. Trusses shall be handled with care prior to erection to avoid damage. Lumber moisture content shall be 19% or less at the time of fabrication, unless noted otherwise (U.N.O.). Connector plates shall be manufactured by Eagle Metal Products (ESR-1082). Plates shall be applied on both faces of truss at each joint. Plate dimensions are listed width x length. Slots (holes) in plate shall run parallel to the plate length. The plate shall be centered on joint and/or placed in accordance with the current version of TPI. Design assumes adequate anchorage will be provided to resist uplift at supports. The seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown. The suitability and use of this component for any particular building design is the responsibility of the building designer, per ANSI/TPI 1-2002 Chapter 2.

TrueBuild® Software v4.05
by Keymark Enterprises, LLC.

| | | | | | | | | | |
|----------------|---------------|----------|---------------|---------------|-----------------|-----------------|-----------|------------------|-------------------|
| SPAN 16-0-0 | PITCH 4/12 | QTY 2 | OHL 0-10-8 | OHR 0-10-8 | CANT L 0-0-0 | CANT R 0-0-0 | PLYS 1 | SPACING 24 in | WGT/PLY 49 lbs |
|----------------|---------------|----------|---------------|---------------|-----------------|-----------------|-----------|------------------|-------------------|



| | | | | | |
|--|--|--|--|---|--------------------------------------|
| Loading Load (psf) TCLL: 105 TCDL: 10 BCLL: 0 BCDL: 10 | General Bldg Code: IRC 2006/ TPI 1-2002 Rep Mbr Increase: No D.O.L.: 115% | CSI Summary TC: 0.66 (6-7) BC: 0.19 (7-8) Web: 0.13 (2-12) | Deflection Vert TL: 0 in Vert LL: 0 in Horz TL: 0 in | L/ (loc) L / 999 (12-1) L / 999 (7-7) 7 | Allowed L / 180 L / 240 |
|--|--|--|--|---|--------------------------------------|

Reaction Summary

| JT | Type | Brg Combo | Brg Width | Max React | Ave React | Max Grav Uplift | Max MWFRS Uplift | Max C&C Uplift | Max Uplift | Max Horiz |
|----|------------|-----------|-----------|-----------|-----------|-----------------|------------------|----------------|------------|-----------|
| 1 | Continuous | 1 | | 813 lbs | 275 plf | | -48 lbs | -239 lbs | -239 lbs | 6 lbs |

Material Summary

TC HF #2 2 x 4
BC HF #2 2 x 4
Webs HF Stud 2 x 4

Bracing Summary

TC Bracing: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC Bracing: Sheathed or purlins at 72" OC, Purlin design by Others.

Loads Summary

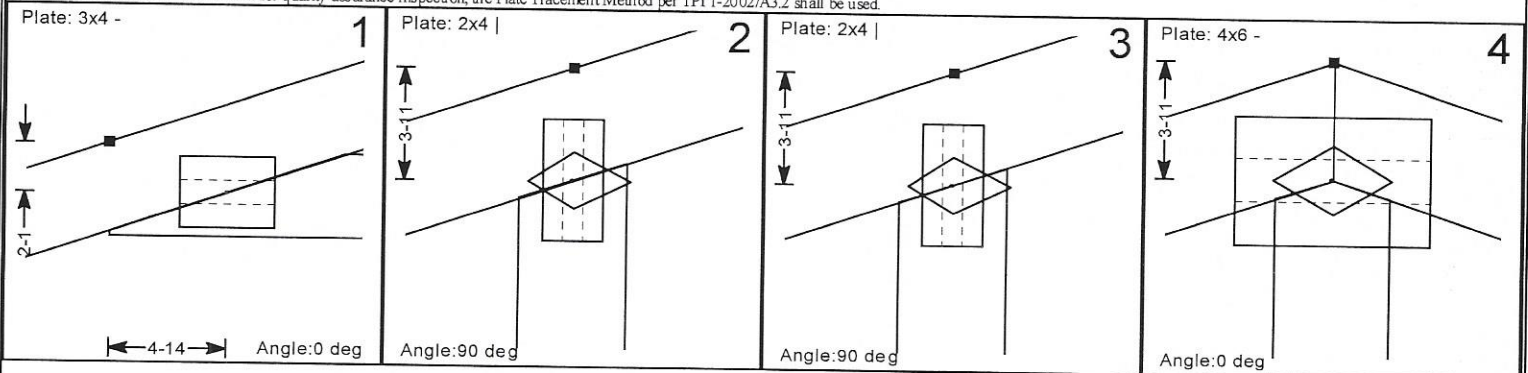
- This truss has been designed for the effects of wind loads in accordance with ASCE 7-05 with the following user defined input: 105 mph, Exposure C, Enclosed, Gable/Hip, Building Category II (I = 1.00), Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- Unbalanced roof live loads have not been considered.

Member Forces Summary

| Member | ID | Force | Member ID | Force | Member ID | Force | Member ID | Force | Member ID | Force | | | |
|--------|------|-------|-----------|-------|-----------|----------|-----------|-------|-----------|-------|-------|---------|------------|
| TC | B-1 | 0.249 | 64 lbs | 2-3 | 0.422 | -115 lbs | 4-5 | 0.215 | 117 lbs | 6-7 | 0.664 | 576 lbs | (-224 lbs) |
| | L-2 | 0.664 | 376 lbs | 3-4 | 0.215 | 117 lbs | 5-6 | 0.422 | -115 lbs | 7-14 | 0.249 | 64 lbs | (-224 lbs) |
| BC | 7-8 | 0.189 | -325 lbs | 9-10 | 0.030 | 108 lbs | 11-12 | 0.059 | 108 lbs | | | | |
| | 8-9 | 0.059 | 108 lbs | 10-11 | 0.030 | 108 lbs | 12-1 | 0.189 | -525 lbs | | | | |
| Webs | 2-12 | 0.128 | -634 lbs | 4-10 | 0.088 | -366 lbs | 6-8 | 0.128 | -634 lbs | | | | |
| | 3-11 | 0.098 | -457 lbs | 5-9 | 0.098 | -457 lbs | | | | | | | |

Notes:

- Gable requires continuous bottom chord bearing.
- Gable webs placed at 24" OC, U.N.O.
- Attach gable webs with 2x4 20ga plates, U.N.O.
- For out-of-plane wind loading, refer to BCSI-B6 published by the WTCA.
- When this truss has been chosen for quality assurance inspection, the Plate Placement Method per TPI 1-2002/A3.2 shall be used.



A copy of this design shall be furnished to the erection contractor. This design is for an individual building component (a truss). It is based on specifications provided by the Truss Designer and performed in accordance with TPI 1-2002 and the 2001 NDS design standard. No responsibility is assumed for the accuracy of information provided by the Truss Designer. Dimensions shall be verified by building designer. Creep deflection is not automatically accounted for by the software. The building designer shall review loading, truss configuration and initial deflection data shown to ensure that this design meets or exceeds minimum loads required by applicable building codes. Compression chords shall be laterally braced by the roof or floor sheathing, directly attached, unless otherwise noted. Bracing shown is for lateral support of individual truss components only to reduce buckling length. It is not wind or lateral load bracing or overall building design bracing which is by others. Refer to BCSI-B3 for recommended truss handling and erection. Do not apply loads beyond weight of erectors until all permanent bracing is in place. Concentration of construction loads greater than the design loads shall not be applied to the trusses at any time. Trusses shall be handled with care prior to erection to avoid damage. Lumber moisture content shall be 19% or less at the time of fabrication, unless noted otherwise (U.N.O.). Connector plates shall be manufactured by Eagle Metal Products (ESR-1082). Plates shall be applied on both faces of truss at each joint. Plate dimensions are listed width x length. Slots (holes) in plate shall run parallel to the plate length. The plate shall be centered on joint and/or placed in accordance with the current version of TPI. Design assumes adequate anchorage will be provided to resist uplift at supports. The seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown. The suitability and use of this component for any particular building design is the responsibility of the building designer, per ANSI/TPI 1-2002 Chapter 2.